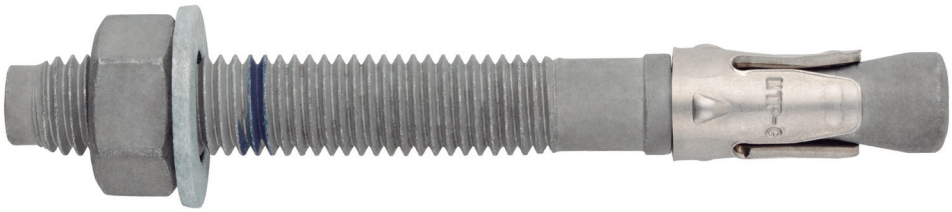
Through-bolt expansion anchor with controlled torque, for use in cracked and non cracked concrete

MTP-G

ETA Assessed option 1. Sherardized shaft. A4 Stainless clip.



PRODUCT INFORMATION

DESCRIPTION
Metallic anchor, with male thread, expansion by controlled torque.

OFFICIAL DOCUMENTATION
- AVCP-1219-CPR-0053.
- ETA 12/0397 option 1.
- Declaration of Performance DoP MTP.

SIZES
M8x50 to M20x200.

DESIGN LOAD RANGE
From 5,6 to 32,8 kN (non-cracked).
From 3,3 to 20,0 kN (cracked).

BASE MATERIAL
Concrete class from C20/25 to C50/60 cracked or non-cracked.

ASSESSMENTS
- Option 1 (Cracked and non-cracked concrete).
- Fire Resistance R30-120.
- Seismic C1 M8÷M20
- Seismic C2 M12÷M20

CHARACTERISTICS AND BENEFITS
- Easy installation.
- Use in cracked and non-craked concrete.
- Use for medium-heavy duty loads.
- Pre-installation or through the drill-hole of the fixture.
- Variety of lengths and diameters: flexibility in assembly.
- For static and quasi-static loads.
- Available at INDEXcal.

MATERIALS
Shaft: Carbon steel, sherardized ≥ 40 µm.
Washer: DIN 125 or DIN 9021, sherardized ≥ 40 µm.
Nut: DIN 934, sherardized ≥ 40 µm.

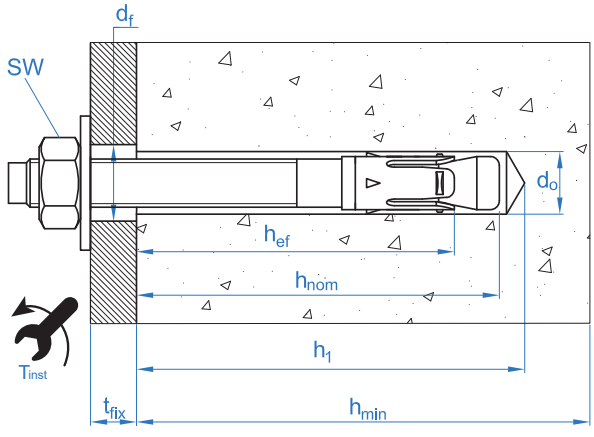
Clip: A4 Stainless steel

APPLICATIONS
- Anchor plates.
- Metallic structures.
- Bridges.
- Urban fitments.
- Protective fences.
- Catenaries.
- Elevators.
- Pipe supports.

MECHANICAL PROPERTIES							
			M8	M10	M12	M16	M20
Cone area section							
A_s	(mm^2)	Cone area section	22,9	41,8	55,4	103,9	176,7
$f_{u,s}$	(N/mm^2)	Chracteristic tension resistance	790	750	730	700	660
$f_{y,s}$	(N/mm^2)	Yield strength	632	600	585	560	530
Threaded area section							
A_s	(mm^2)	Cone area section	36,6	58,0	84,3	157,0	245,0
$f_{u,s}$	(N/mm^2)	Chracteristic tension resistance	600	600	600	600	600
$f_{y,s}$	(N/mm^2)	Yield Strength	480	480	480	480	480

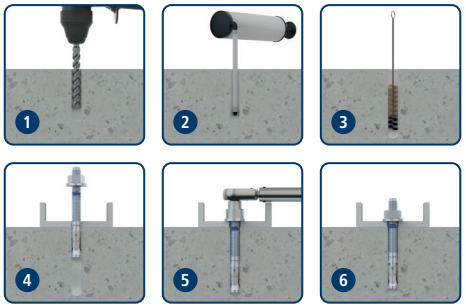
INSTALLATION DATA							
SIZE			M8	M10	M12	M16	M20
Code			APG08XXX	APG10XXX	APG12XXX	APG16XXX	APG20XXX
d_0	Nominal diameter of drill bit	[mm]	8	10	12	16	20
T_{ins}	Installation torque moment	[Nm]	15	40	60	100	200
$d_f \leq$	Diameter of clearance hole in the fixture	[mm]	9	12	14	18	22
h_1	Minimum drill hole depth	[mm]	60	75	85	105	125
h_{nom}	Installation depth	[mm]	55	68	80	97	114
h_{ef}	Effective embedment depth	[mm]	48	60	70	85	100
h_{min}	Minimum base material thickness	[mm]	100	120	140	170	200
t_{fix}	Maximum thickness of fixture	[mm]	L - 66	L - 80	L - 96	L - 117	L-138
$s_{cr,N}$	Critical spacing	[mm]	144	180	210	255	300
$c_{cr,N}$	Critical edge distance	[mm]	72	90	105	128	150
$s_{cr,sp}$	Critical distance (splitting)	[mm]	288	300	350	510	600
$c_{cr,sp}$	Critical edge distance (splitting)	[mm]	144	150	175	255	300
s_{min}	Minimum spacing	[mm] [mm]	40 55	40 70	60 75	65 95	95 105
c_{min}	Minimum edge distance	[mm] [mm]	45 55	45 90	55 110	70 115	95 105
SW	Installation wrench		13	17	19	24	30

*L = Total anchor length



Code	INSTALLATION PRODUCTS
	Hammer drill
BHDSXXXXX	Concrete Drill bits
MOBOMBA	Blow pump
MORCEPKIT	Cleaning Brush
DOMTAXX	Installation hammering tool
	Torque wrench
	Hexagonal socket

INSTALLATION



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Resistances in C20/25 concrete for an isolated anchor, without effects of edge distance or spacing

Characteristic Resistance N_{Rk} and V_{Rk}

TENSION								SHEAR							
Size			M8	M10	M12	M16	M20	Size			M8	M10	M12	M16	M20
N_{Rk}	Non-cracked concrete	[kN]	10,0	18,0	28,8	36,0	49,2	V_{Rk}	Non-cracked concrete	[kN]	11,0	17,4	25,3	47,1	73,1
N_{Rk}	Cracked concrete	[kN]	6,0	10,0	16,0	27,0	30,0	V_{Rk}	Cracked concrete	[kN]	11,5	17,4	25,3	53,9	68,8

Design Resistance N_{Rd} and V_{Rd}

TENSION								SHEAR							
Size			M8	M10	M12	M16	M20	Size			M8	M10	M12	M16	M20
N_{Rd}	Non-cracked concrete	[kN]	5,6	12,0	19,2	24,0	32,8	V_{Rd}	Non-cracked concrete	[kN]	8,8	13,9	20,2	37,7	58,5
N_{Rd}	Cracked concrete	[kN]	3,3	6,7	10,7	18,0	20,0	V_{Rd}	Cracked concrete	[kN]	7,6	13,9	20,2	35,9	45,9

Maximum Loads Recommended N_{rec} and V_{rec}

TENSION								SHEAR							
Size			M8	M10	M12	M16	M20	Size			M8	M10	M12	M16	M20
N_{rec}	Non-cracked concrete	[kN]	4,0	8,6	13,7	17,1	23,4	V_{rec}	Non-cracked concrete	[kN]	6,3	9,9	14,5	26,9	41,8
N_{rec}	Cracked concrete	[kN]	2,4	4,8	7,6	12,9	14,3	V_{rec}	Cracked concrete	[kN]	5,4	9,9	14,5	25,7	32,8

Simplified calculation method

European Technical Assessment ETA 12/0397

Simplified version of the calculation method according to Eurocode 2 EN 1992-4. Resistance is calculated according to the data shown in assessment 12/0397.

- Influence of concrete strength.
- Influence of edge distance.
- Influence of spacing between anchors.
- Influence of reinforcements.
- Influence of base material thickness.
- Influence of load application angle.
- Valid for a group of two anchors.

The calculation method is based on the following simplification: **Different loads do not act on individual anchors, without eccentricity.**

INDEXcal

For a more accurate calculation and to take more constructive provisions into account, we recommend using our calculation program INDEXcal. It may be easily downloaded from our website **www.indexfix.com**

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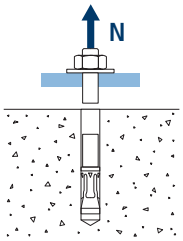
TENSION LOADS

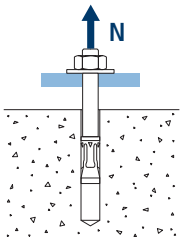
- Steel design resistance: $N_{Rd,s}$
- Pull-out design resistance: $N_{Rd,p} = N^{o}_{Rd,p} \cdot \Psi_{c}$
- Concrete cone design resistance: $N_{Rd,c} = N^{o}_{Rd,c} \cdot \Psi_{b} \cdot \Psi_{s,N} \cdot \Psi_{c,N} \cdot \Psi_{re,N}$
- Concrete splitting design resistance: $N_{Rd,sp} = N^{o}_{Rd,c} \cdot \Psi_{b} \cdot \Psi_{s,sp} \cdot \Psi_{c,sp} \cdot \Psi_{re,N} \cdot \Psi_{h,sp}$

Steel Design resistance						
$N_{Rd,s}$						
Size			M8	M10	M12	M16	M20
N^{o}_{Rd}		[kN]	12,1	20,9	26,9	48,5	77,7



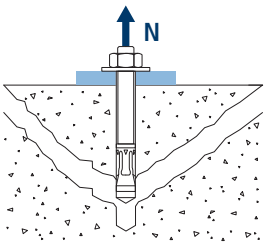
Pull-out design resistance							
$N_{Rd,p} = N^{o}_{Rd,p} \cdot \Psi_{c}$							
Size			M8	M10	M12	M16	M20
$N^{o}_{Rd,p}$	Non-cracked concrete	[kN]	5,56	12,00	-*	24,00	-*
$N^{o}_{Rd,p}$	Cracked concrete	[kN]	3,33	6,67	10,67	-*	20,00

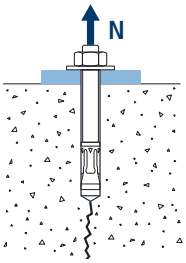
* Pull-out failure is not decisive.



Concrete cone design resistance							
$N_{Rd,c} = N^{o}_{Rd,c} \cdot \Psi_{b} \cdot \Psi_{s,N} \cdot \Psi_{c,N} \cdot \Psi_{re,N}$							
Concrete splitting design resistance*							
$N_{Rd,sp} = N^{o}_{Rd,c} \cdot \Psi_{b} \cdot \Psi_{s,sp} \cdot \Psi_{c,sp} \cdot \Psi_{re,N} \cdot \Psi_{h,sp}$							
Size			M8	M10	M12	M16	M20
$N^{o}_{Rd,c}$	Non-cracked concrete	[kN]	9,1	15,2	19,2	25,7	32,8
$N^{o}_{Rd,c}$	Cracked concrete	[kN]	6,4	10,7	13,5	18,0	23,0

*Concrete splitting design resistance must only be considered for non-cracked concrete.





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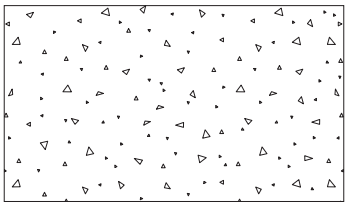
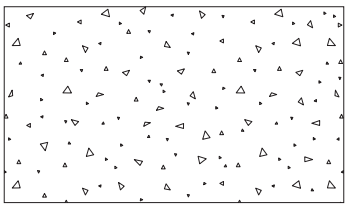
Coefficients of influence

Influence of concrete strength resistance in pul-out failure Ψ_c						
		M8	M10	M12	M16	M20
Ψ_c	C 20/25			1,00		
	C 30/37	1,22	1,17	1,22	1,22	1,17
	C 40/50	1,41	1,31	1,41	1,41	1,31
	C 50/60	1,58	1,43	1,58	1,58	1,43



Influence of concrete strength in concret cone and splitting failure Ψ_b						
		M8	M10	M12	M16	M20
Ψ_b	C 20/25			1,00		
	C 30/37			1,22		
	C 40/50			1,41		
	C 50/60			1,58		



$$\Psi_b = \sqrt{\frac{f_{ck,cube}}{25}} \geq 1$$

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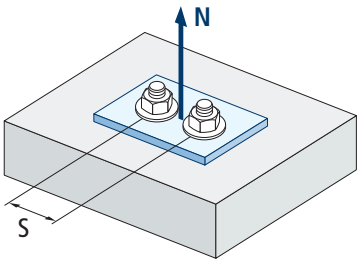

$$\Psi_{s,N} = 0{,}5 + \frac{s}{2 \cdot S_{cr,N}} \leq 1$$

Influence of spacing (concrete cone) $\Psi_{s,N}$					
s [mm]	MTP-G				
	M8	M10	M12	M16	M20
40	0,64	0,61	Invalid value		
50	0,67	0,64			
55	0,69	0,65			
60	0,71	0,67	0,64		
65	0,73	0,68	0,65	0,63	
70	0,74	0,69	0,67	0,64	
80	0,78	0,72	0,69	0,66	
85	0,80	0,74	0,70	0,67	
90	0,81	0,75	0,71	0,68	
95	0,83	0,76	0,73	0,69	
100	0,85	0,78	0,74	0,70	
105	0,86	0,79	0,75	0,71	
110	0,88	0,81	0,76	0,72	
120	0,92	0,83	0,79	0,74	
125	0,93	0,85	0,80	0,75	0,71
126	0,94	0,85	0,80	0,75	0,71
128	0,94	0,86	0,80	0,75	0,71
130	0,95	0,86	0,81	0,75	0,72
135	0,97	0,88	0,82	0,76	0,73
144	1,00	0,90	0,84	0,78	0,74
150		0,92	0,86	0,79	0,75
165		0,96	0,89	0,82	0,78
170		0,97	0,90	0,83	0,78
180		1,00	0,93	0,85	0,80
195			0,96	0,88	0,83
200			0,98	0,89	0,83
210			1,00	0,91	0,85
220				0,93	0,87
225				0,94	0,88
252				0,99	0,92
255	Value without reduction = 1			1,00	0,93
260					0,93
300					1,00

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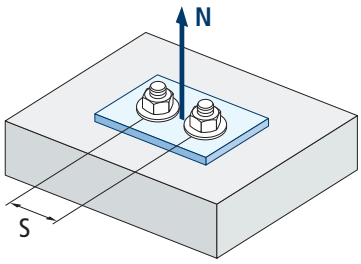
Influence of spacing (concrete splitting) $\Psi_{s,sp}$					
s [mm]	MTP-G				
	M8	M10	M12	M16	M20
40	0,57	0,57	Invalid value		
50	0,59	0,58			
55	0,60	0,59			
60	0,60	0,60	0,59		
65	0,61	0,61	0,59	0,56	
70	0,62	0,62	0,60	0,57	
80	0,64	0,63	0,61	0,58	
85	0,65	0,64	0,62	0,58	
90	0,66	0,65	0,63	0,59	
95	0,66	0,66	0,64	0,59	0,58
100	0,67	0,67	0,64	0,60	0,58
110	0,69	0,68	0,66	0,61	0,59
125	0,72	0,71	0,68	0,62	0,60
128	0,72	0,71	0,68	0,63	0,61
135	0,73	0,73	0,69	0,63	0,61
140	0,74	0,73	0,70	0,64	0,62
150	0,76	0,75	0,71	0,65	0,63
160	0,78	0,77	0,73	0,66	0,63
165	0,79	0,78	0,74	0,66	0,64
168	0,79	0,78	0,74	0,66	0,64
180	0,81	0,80	0,76	0,68	0,65
192	0,83	0,82	0,77	0,69	0,66
200	0,85	0,83	0,79	0,70	0,67
210	0,86	0,85	0,80	0,71	0,68
220	0,88	0,87	0,81	0,72	0,68
260	0,95	0,93	0,87	0,75	0,72
280	0,99	0,97	0,90	0,77	0,73
288	1,00	0,98	0,91	0,78	0,74
300		1,00	0,93	0,79	0,75
336			0,98	0,83	0,78
350			1,00	0,84	0,79
360				0,85	0,80
412				0,90	0,84
425				0,92	0,85
500				0,99	0,92
510	Value without reduction = 1			1,00	0,93
560					0,97
600					1,00



$$\Psi_{s,sp} = 0,5 + \frac{s}{2 \cdot S_{cr,sp}} \leq 1$$

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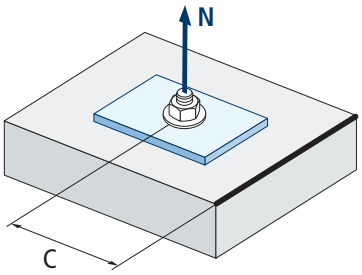

$$\Psi_{c,sp} = 0{,}35 + \frac{0{,}5 \cdot c}{C_{cr,sp}} + \frac{0{,}15 \cdot c^2}{C_{cr,sp}^{\ 2}} \leq 1$$

Influence of concrete edge distance (splitting) $\Psi_{c,sp}$					
c [mm]	MTP-G				
	M8	M10	M12	M16	M20
45	0,52	0,51	Invalid value		
50	0,54	0,53			
55	0,56	0,55	0,52		
60	0,58	0,57	0,54		
65	0,61	0,59	0,56		
70	0,63	0,62	0,57	0,50	
75	0,65	0,64	0,59	0,51	
80	0,67	0,66	0,61	0,52	
83	0,69	0,67	0,62	0,53	
84	0,69	0,68	0,62	0,53	
85	0,70	0,68	0,63	0,53	
90	0,72	0,70	0,65	0,55	
95	0,75	0,73	0,67	0,56	0,52
100	0,77	0,75	0,68	0,57	0,53
105	0,79	0,77	0,70	0,58	0,54
110	0,82	0,80	0,72	0,59	0,55
125	0,90	0,87	0,78	0,63	0,58
128	0,91	0,89	0,80	0,64	0,59
130	0,92	0,90	0,80	0,64	0,59
135	0,95	0,92	0,82	0,66	0,61
140	0,98	0,95	0,85	0,67	0,62
144	1,00	0,97	0,86	0,68	0,62
150		1,00	0,89	0,70	0,64
168			0,97	0,74	0,68
175			1,00	0,76	0,69
180				0,78	0,70
206				0,85	0,76
213				0,87	0,78
250				0,98	0,87
255	Value without reduction = 1			1,00	0,88
280					0,95
300					1,00

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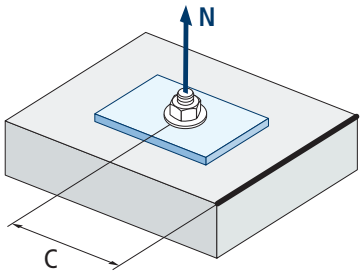
Influence of concrete edge distance (concrete cone) $\Psi_{c,N}$

c [mm]	MTP-G				
	M8	M10	M12	M16	M20
40	0,67	0,60	Invalid value		
50	0,77	0,67			
53	0,80	0,70			
60	0,87	0,75			
63	0,90	0,77			
65	0,92	0,79	0,72		
70	0,98	0,83	0,75		
72	1,00	0,85	0,76		
75		0,87	0,78		
80		0,91	0,82		
83		0,94	0,84		
85		0,96	0,85		
90		1,00	0,89		
95			0,93	0,80	
100			0,96	0,83	
105			1,00	0,86	
110				0,89	
113				0,91	
125				0,98	0,87
126				0,99	0,88
128	Value without reduction = 1			1,00	0,89
135					0,92
150					1,00

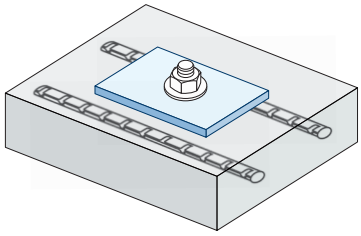


$$\Psi_{c,N} = 0{,}35 + \frac{0{,}5 \cdot c}{C_{cr,N}} + \frac{0{,}15 \cdot c^2}{C_{cr,N}^{\ 2}} \leq 1$$

Influence of reinforcements $\Psi_{re,N}$

$\Psi_{re,N}$	MTP-G				
	M8	M10	M12	M16	M20
	0,74	0,80	0,85	0,93	1,00

*This factor only applies for a high density of reinforcements. If in the area of the anchor there are reinforcements with a distancing of ≥ 150 mm (any diameter) or with a diameter ≤ 10 mm and a distancing of ≥ 100 mm, a $f_{re,N}$ = 1 factor may be applied.

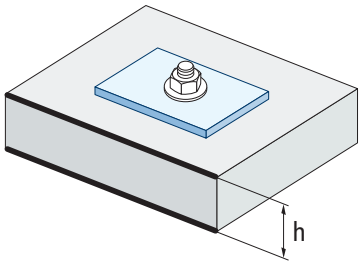


$$\Psi_{re,N} = 0{,}5 + \frac{h_{ef}}{200} \leq 1$$

Influence of base material thickness $\Psi_{h,sp}$

$\Psi_{h,sp}$	MTP-G									
h/hef	2,00	2,20	2,40	2,60	2,80	3,00	3,20	3,40	3,60	≥3,68
$\Psi_{h,sp}$	1,00	1,07	1,13	1,19	1,25	1,31	1,37	1,42	1,48	1,50

$$\Psi_{h,sp} = \left(\frac{h}{2 \cdot h_{ef}}\right)^{2/3} \leq 1{,}5$$

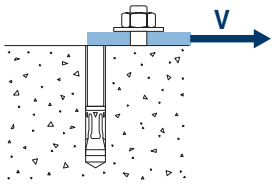


MTP-G

SHEAR LOADS

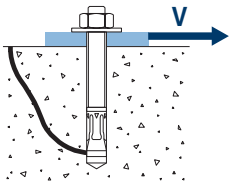
- Steel design resistance without lever arm: $V_{Rd,s}$
- Pry-out design resistance: $V_{Rd,cp} = k \cdot N^{o}_{Rd,c}$
- Concrete edge design resistance: $V_{Rd,c} = V^{o}_{Rd,c} \cdot \Psi_{b} \cdot \Psi_{se,V} \cdot \Psi_{c,V} \cdot \Psi_{re,V} \cdot \Psi_{\alpha,V} \cdot \Psi_{h,V}$

Steel design resistance						
$V_{Rd,s}$						
Size		M8	M10	M12	M16	M20
$V_{Rd,s}$	[kN]	8,8	13,9	20,2	37,7	58,5

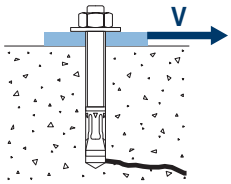


Pry-out design resistance*						
$V_{Rd,cp} = k \cdot N^{o}_{Rd,c}$						
Size		M8	M10	M12	M16	M20
k		1	2	2	2	2

* $N^{o}_{Rd,c}$ Concrete cone design resistance for tension loads



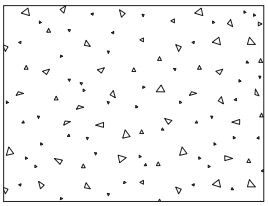
Concrete edge resistance							
$V_{Rd,c} = V^{o}_{Rd,c} \cdot \Psi_{b} \cdot \Psi_{se,V} \cdot \Psi_{c,V} \cdot \Psi_{re,V} \cdot \Psi_{\alpha,V} \cdot \Psi_{h,V}$							
Size			M8	M10	M12	M16	M20
$V^{o}_{Rd,c}$	Non-cracked concrete	[kN]	6,2	8,9	11,5	15,9	20,8
	Cracked concrete	[kN]	4,4	6,3	8,2	11,3	14,7



MTP-G

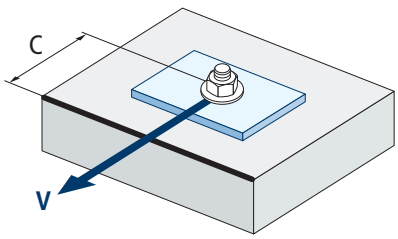
Coefficients of influence

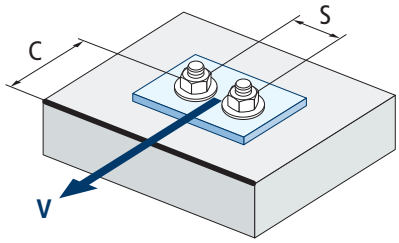
Influence of concrete strength in concrete edge failure Ψ_b						
		M8	M10	M12	M16	M20
Ψ_b	C 20/25	1,00				
	C 30/37	1,22				
	C 40/50	1,41				
	C 50/60	1,55				



$$\Psi_b = \sqrt{\frac{f_{ck,cube}}{25}} \geq 1$$

Influence of edge distance and spacing $\Psi_{se,V}$																	
FOR ONE ANCHOR ONLY																	
c/h_{ef}	0,50	0,75	1,00	1,25	1,50	1,75	2,00	2,25	2,50	2,75	3,00	3,25	3,50	3,75	4,00	4,50	5,00
Isolated	0,35	0,65	1,00	1,40	1,84	2,32	2,83	3,38	3,95	4,56	5,20	5,86	6,55	7,26	8,00	9,55	11,18
FOR TWO ANCHORS																	
s/c	c/h_{ef}	0,50	0,75	1,00	1,25	1,50	1,75	2,00	2,25	2,50	2,75	3,00	3,25	3,50	3,75	4,00	4,50	5,00
	1,0	0,24	0,43	0,67	0,93	1,22	1,54	1,89	2,25	2,64	3,04	3,46	3,91	4,37	4,84	5,33	6,36	7,45
	1,5	0,27	0,49	0,75	1,05	1,38	1,74	2,12	2,53	2,96	3,42	3,90	4,39	4,91	5,45	6,00	7,16	8,39
	2,0	0,29	0,54	0,83	1,16	1,53	1,93	2,36	2,81	3,29	3,80	4,33	4,88	5,46	6,05	6,67	7,95	9,32
	2,5	0,32	0,60	0,92	1,28	1,68	2,12	2,59	3,09	3,62	4,18	4,76	5,37	6,00	6,66	7,33	8,75	10,25
	≥3,0	0,35	0,65	1,00	1,40	1,84	2,32	2,83	3,38	3,95	4,56	5,20	5,86	6,55	7,26	8,00	9,55	11,18

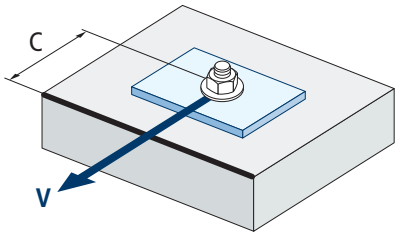




$$\Psi_{se,V} = \left(\frac{c}{h_{ef}}\right)^{1,5}$$

$$\Psi_{se,V} = \left(\frac{c}{h_{ef}}\right)^{1,5} \cdot \left(1 + \frac{s}{3 \cdot c}\right) \cdot 0,5 \leq \left(\frac{c}{h_{ef}}\right)^{1,5}$$

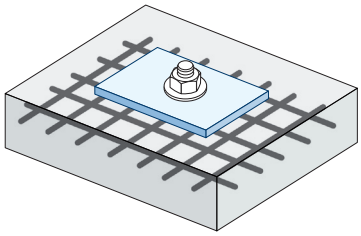
MTP-G



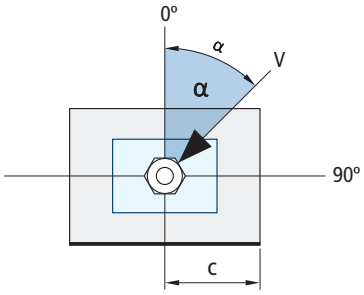
$$\Psi_{c,V} = \left(\frac{d}{c}\right)^{0,20}$$

Influence of concrete edge distance $\Psi_{c,V}$					
c [mm]	MTP-G				
	M8	M10	M12	M16	M20
40					
45					
50	0,69	Invalid value			
55	0,68				
60	0,67	0,70			
65	0,66	0,69	0,71		
70	0,65	0,68	0,70		
80	0,63	0,66	0,68		
85	0,62	0,65	0,68	0,72	
90	0,62	0,64	0,67	0,71	
100	0,60	0,63	0,65	0,69	0,72
105	0,60	0,62	0,65	0,69	0,72
110	0,59	0,62	0,64	0,68	0,71
120	0,58	0,61	0,63	0,67	0,70
125	0,58	0,60	0,63	0,66	0,69
130	0,57	0,60	0,62	0,66	0,69
135	0,57	0,59	0,62	0,65	0,68
140	0,56	0,59	0,61	0,65	0,68
150	0,56	0,58	0,60	0,64	0,67
160	0,55	0,57	0,60	0,63	0,66
170	0,54	0,57	0,59	0,62	0,65
175	0,54	0,56	0,59	0,62	0,65
180	0,54	0,56	0,58	0,62	0,64
190	0,53	0,55	0,58	0,61	0,64
200	0,53	0,55	0,57	0,60	0,63
210	0,52	0,54	0,56	0,60	0,62
220	0,52	0,54	0,56	0,59	0,62
230	0,51	0,53	0,55	0,59	0,61
240	0,51	0,53	0,55	0,58	0,61
250	0,50	0,53	0,54	0,58	0,60
260	0,50	0,52	0,54	0,57	0,60
270	0,49	0,52	0,54	0,57	0,59
280	0,49	0,51	0,53	0,56	0,59
290	0,49	0,51	0,53	0,56	0,59
300	0,48	0,51	0,53	0,56	0,58

Influence of reinforcements $\Psi_{re,V}$			
	Without perimetral reinforcements	Perimetral reinforcements ≥ Ø12 mm	Perimetral reinforcements with brackets ≤ 100 mm
Non-cracked concrete	1	1	1
Cracked concrete	1	1,2	1,4

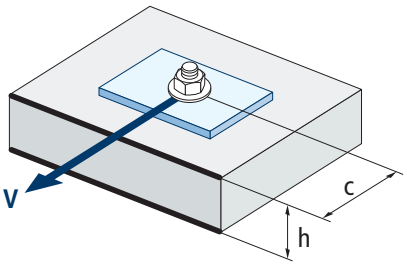


Influence of load application angle $\Psi_{\alpha,V}$										
Angle, α(°)	0°	10°	20°	30°	40°	50°	60°	70°	80°	90°
$\Psi_{\alpha,V}$	1,00	1,01	1,05	1,13	1,24	1,40	1,64	1,97	2,32	2,50



$$\Psi_{\alpha,V} = \sqrt{\frac{1}{(\cos \alpha_v)^2 + \left(\frac{\sin \alpha_v}{2,5}\right)^2}} \geq 1$$

Influence of base material thickness $\Psi_{h,V}$										
	MTP-G									
h/c	0,15	0,30	0,45	0,60	0,75	0,90	1,05	1,20	1,35	≥1,5
$\Psi_{h,V}$	0,32	0,45	0,55	0,63	0,71	0,77	0,84	0,89	0,95	1,00



$$\Psi_{h,V} = \left(\frac{h}{1,5 \cdot c}\right)^{0,5} \geq 1,0$$

MTP-G

FIRE RESISTANCE

Characteristic Resistance*										
	TENSION					SHEAR				
	M8	M10	M12	M16	M20	M8	M10	M12	M16	M20
RF30	0,4	0,9	1,7	3,1	4,9	0,4	0,9	1,7	3,1	4,9
RF60	0,3	0,8	1,3	2,4	3,7	0,3	0,8	1,3	2,4	3,7
RF90	0,3	0,6	1,1	2,0	3,2	0,3	0,6	1,1	2,0	3,2
RF120	0,2	0,5	0,8	1,6	2,5	0,2	0,5	0,8	1,6	2,5

*The safety factor for design resistance under fire exposure is $\gamma_{M,fi}$=1 (in abscence of other national regulations). As a eresult the Characteristic Resistance is the same as Design Resistance.

Maximum Load Recommended										
	TENSION					SHEAR				
	M8	M10	M12	M16	M20	M8	M10	M12	M16	M20
RF30	0,3	0,6	1,2	2,2	3,5	0,3	0,6	1,2	2,2	3,5
RF60	0,2	0,6	0,9	1,7	2,6	0,2	0,6	0,9	1,7	2,6
RF90	0,2	0,4	0,8	1,4	2,3	0,2	0,4	0,8	1,4	2,3
RF120	0,1	0,4	0,6	1,1	1,8	0,1	0,4	0,6	1,1	1,8

RANGE

Code	Size	Maximum thickness of fixture	Axle letter (length)		
• APG06060	M6 x 60 Ø6	10	B	200	1.200
• APG06070	M6 x 70 Ø6	20	C	200	1.200
• APG06100	M6 x 100 Ø6	50	E	200	800
• APG08050	M8 x 50 Ø8	2	A	100	800
• APG08060	M8 x 60 Ø8	12	B	100	800
APG08075	M8 x 75 Ø8	9	C	100	600
APG08095	M8 x 95 Ø8	29	E	100	600
APG08115	M8 x 115 Ø8	49	G	100	400
• APG10070	M10 x 70 Ø10	5	C	100	400
APG10090	M10 x 90 Ø10	10	E	100	400
APG10105	M10 x 105 Ø10	25	F	50	300
APG10115	M10 x 115 Ø10	35	G	50	200
APG10135	M10 x 135 Ø10	55	H	50	200
APG10165	M10 x 165 Ø10	85	K	50	200
APG10185	M10 x 185 Ø10	105	L	50	150

Code	Size	Maximum thickness of fixture	Axle letter (length)		
• APG12080	M12 x 80 Ø12	4	D	50	300
APG12110	M12 x 110 Ø12	14	F	50	200
APG12130	M12 x 130 Ø12	34	H	50	200
APG12150	M12 x 150 Ø12	54	I	50	100
APG12160	M12 x 160 Ø12	64	J	50	100
APG12180	M12 x 180 Ø12	84	L	50	150
APG12200	M12 x 200 Ø12	104	M	50	150
APG16125	M16 x 125 Ø16	8	G	25	100
APG16145	M16 x 145 Ø16	28	I	25	100
APG16175	M16 x 175 Ø16	58	K	25	50
APG16220	M16 x 220 Ø16	103	O	25	50
APG16250	M16 x 250 Ø16	133	Q	25	50
APG20170	M20 x 170 Ø20	32	K	20	40
APG20200	M20 x 200 Ø20	62	M	20	40

• Non assessed sizes. Resistance values and installation data are not applicable to these references. For further information, please contact Technical Department.